

## Initial Inflow Design Flood Control System Plan for Water Recycle Ponds

Entergy Arkansas, Inc.
White Bluff Plant
Redfield, Jefferson County, Arkansas

October 2018

Prepared For Entergy Arkansas, Inc.

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TRC Environmental Corporation | Entergy Arkansas, Inc. Initial Inflow Design Flood Control System Plan for Water Recycle Ponds Entergy White Bluff Plant, Redfield, Jefferson County, Arkansas

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Revision Number	Revision Date	Section Revised	Summary of Revisions

## Section 1 **Background**

Entergy Arkansas, Inc. (Entergy) operates the Entergy White Bluff Plant (Plant), located at 1100 White Bluff Road, Redfield, AR 72132. This Plant operates two Water Recycle Ponds: A and B (Ponds), as part of its process water system for bottom ash transport. Pursuant to United States Environmental Protection Agency (USEPA) Disposal of Coal Combustion Residuals (CCR) From Electric Utilities Final Rule (CCR Rule) Section 40 Code of Federal Regulations (CFR) § 257.82, this Initial Inflow Design Flood Control System Plan (Plan) presents the design and construction features of the Ponds that adequately manage and maintain the inflow design flood control system.

#### 1.1 Documents Reviewed

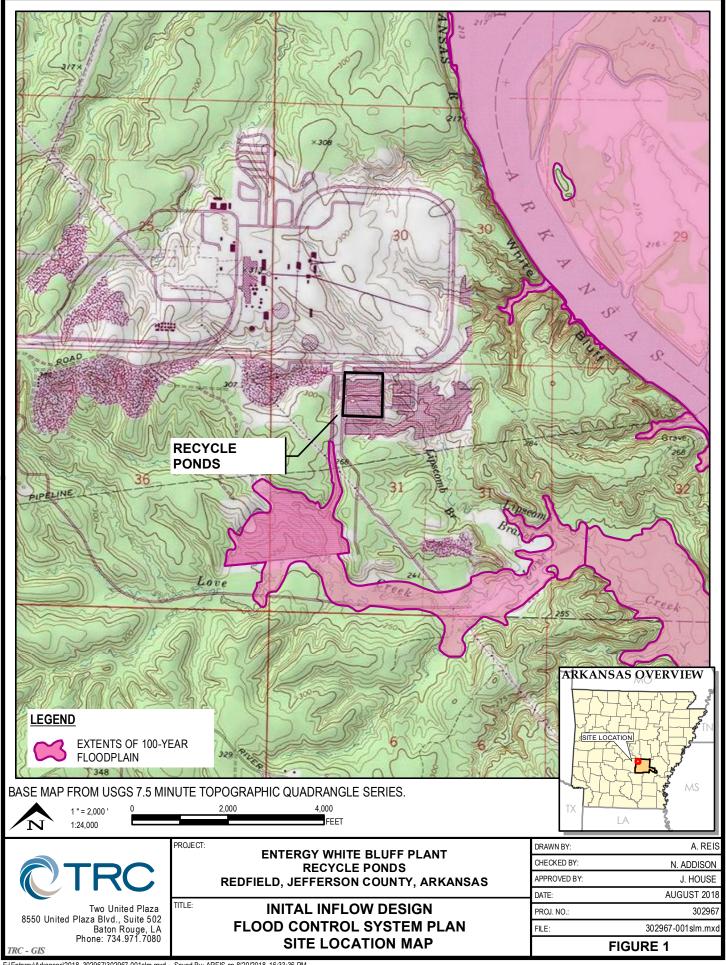
To develop this plan the following documents were reviewed by TRC:

- Design drawings by Chas. T. Main, Inc. for the White Bluff Steam Electric Station: 1977-1980 – 2800MW (Net) Installation – Units 1 Thru 4: Surge Pond Overflow:
  - Holding Pond No. 2 and Water Treating Area Plan. Rev. 8, 1/24/83. Drawing 3022-00
  - Holding Pond No. 2 and Water Treating Area Section and Details. Rev. 2, 10/27/81. Drawing No. 3022-01
- Entergy's Flow Diagram drawing for Ash Disposal for White Bluff Units 1, 2, 3, and 4. Rev. 0, 1/4/95. Drawing 2028-00
- Topographic Survey performed by B&F Engineering, Inc. 10/4/18
  - Recycle Ponds Plan View. Drawing EX 1/3
  - Recycle Ponds Profiles. Drawings EX 2/3, EX 3/3

#### 1.2 **Existing Conditions**

The Plant is located near Redfield, Jefferson County, Arkansas (Figure 1). The Plant is located at approximate latitude 34°25′11.4″ N, longitude 92°9′20.60″ W (front gate). The Ponds are located to the southeast of the Plant. Pond A has an approximate surface area of 7.0 acres and Pond B has an approximate surface area of 6.5 acres. The typical water level elevation in each pond is approximately 278 ft North American Vertical Datum of 1988 (NAVD88) based on field observations during June 2018. Topography surrounding the immediate vicinity of the Ponds is generally flat-lying, with existing ground surface elevations ranging from approximately 277 to 285 ft NAVD88. Natural topography in the vicinity of the ponds is gently to steeply sloping.

Water contained in the Ponds is part of the facility's bottom ash transport system. There are storm water swales around the Ponds to control storm water and limit the volume of run-on. The natural ground beyond the ponds gently slopes to the southeast. A topographic survey completed by B&F Engineering, Inc. in October 2018 confirmed the elevations of the top of berm and surrounding areas.



# Section 2 Inflow Design Flood Control

40 CFR § 257.82 specifies requirements for hydrologic and hydraulic capacity of CCR units. The purpose of this report is to demonstrate the hydrologic and hydraulic capacity of the Ponds meet the requirement of the CCR Rule. Per ERM's Report, Pond A was classified as low hazard in accordance with 40 CFR § 57.73 due to the low berm height and the anticipated capacity of the adjacent plant water retention structures on the Plant property to safely receive a discharge from a potential berm failure. Pond B is incised, and a hazard potential classification is not required per 40 CFR § 57.73(a) according to the same report. Based on the low hazard potential classification for Pond A, both ponds were evaluated to determine if the flow into the units during and following the peak discharge of the 100-year flood was adequately managed [40 CFR § 257.82(a)(3)(iii)].

The Ponds were designed and utilized as part of the Plant's bottom ash transport water system. The design meets the requirements of 40 CFR § 257.82(a)(1), (2) and (3), which states:

- (1) The inflow design flood control system must adequately manage flow into the CCR unit during and following the peak discharge of the inflow design flood specified in paragraph (a)(3) of this section.
- (2) The inflow design flood control system must adequately manage flow from the CCR unit to collect and control the peak discharge resulting from the inflow design flood specified in paragraph (a)(3) of this section.
- (3) The inflow design flood is:
  - (i) For a high hazard potential CCR surface impoundment, as determined under 40 CFR § 257.73(a)(2) or 40 CFR § 257.74(a)(2), the probable maximum flood;
  - (ii) For a significant hazard potential CCR surface impoundment, as determined under 40 CFR § 257.73(a)(2) or 40 CFR § 257.74(a)(2), the 1000-year flood;
  - (iii) For a low hazard potential CCR surface impoundment, as determined under 40 CFR §257.73(a)(2) or 40 CFR § 257.74(a)(2), the 100-year flood (Pond A);
  - (iv) For an incised CCR surface impoundment, the 25-year flood (Pond B).

The Flood Insurance Rate Map for the Plant (Federal Emergency Management Agency, 2009) indicates that the Ponds are located in an area determined to be outside the 0.2 percent annual chance flood, refer to Appendix B. Figure 2 shows the extents of the 100-year flood. The Ponds

TRC Environmental Corporation | Entergy Arkansas, Inc. Initial Inflow Design Flood Control System Plan for Water Recycle Ponds Entergy White Bluff Plant, are located in an area that is above the 100-year flood elevation; therefore, a dedicated system to control flood waters is not required. The Ponds have been designed with several inflow control features presented in the sections below to mitigate and control stormwater inflows during a 100-year storm event.

In addition to the flood control requirements above, 40 CFR § 257.82(b) addresses discharges:

Discharge from the CCR unit must be handled in accordance with the surface water requirements under §257.3-3.

The Ponds were designed and utilized as part of the Plant's bottom ash transport water system and comply with the requirements of 40 CFR § 257.82(b). Water discharges from the Plant are permitted through the Arkansas Department of Environmental Quality (ADEQ) for the National Pollutant Discharge Elimination System.

#### 2.1 **Recycle Ponds Operation**

#### 2.1.1 **Recycle Pond-Current Operation**

Site infrastructure enables the Ponds to be operated either singularly, in parallel, or in series. Each pond has a 24" inlet pipe from the Plant. Additionally, Pond A has an inlet from an overflow weir in Pond B at elevation 273 ft NAVD88. The Ponds can be drained via pumps in a pump house located between the ponds with a 30-inch intake pipe from each pond. The Ponds were designed to operate with 2 feet of freeboard, height difference from the top of the surrounding ground surface to the design water level.

The following control measures are implemented during operation to control the water levels in the Ponds:

- An overflow weir at elevation 273 ft NAVD88 in Pond B that drains to Pond A.
- The ability to divert flow from Pond B to Pond A if water levels become higher than anticipated.
- Operate pumps as needed to control the pond water levels.
- An 18-inch diameter emergency overflow pipe at elevation 279.65 ft NAVD88 that drains to the Surge Pond.
- Regularly check and maintain grades surrounding the ponds to minimize the area contributing to storm water run-on.

### 2.1.2 Recycle Ponds – Pond A Closure

It is anticipated that Pond A (south pond) will be closed by removal of CCR. The following control measure will be implemented during operation of Pond B to control the water level:

- Operate pumps as needed to control the pond water levels.
- An 18-inch diameter emergency overflow pipe at an appropriate elevation that drains to the Surge Pond.
- Regularly check and maintain grades surrounding Pond B to minimize the area contributing to storm water run-on.

### 2.2 Run-on Control

As designed, the Ponds meet the requirement to control the storm water run-on from a 100-year, 24-hour storm event based upon the Precipitation Frequency Estimates from the National Oceanic and Atmospheric Administration. As designed, the Ponds meet the requirement to operate with at least two feet of freeboard, height from the design water elevation to the top of berm elevation. The storm water run-on volume calculated for the design storm was compared to the storage capacity above the Ponds design operating elevation. The evaluation determined that there is sufficient capacity in the Ponds when operating with two feet of freeboard to accept run-on volume from a 100-year, 24-hour storm event, refer to Appendix B.

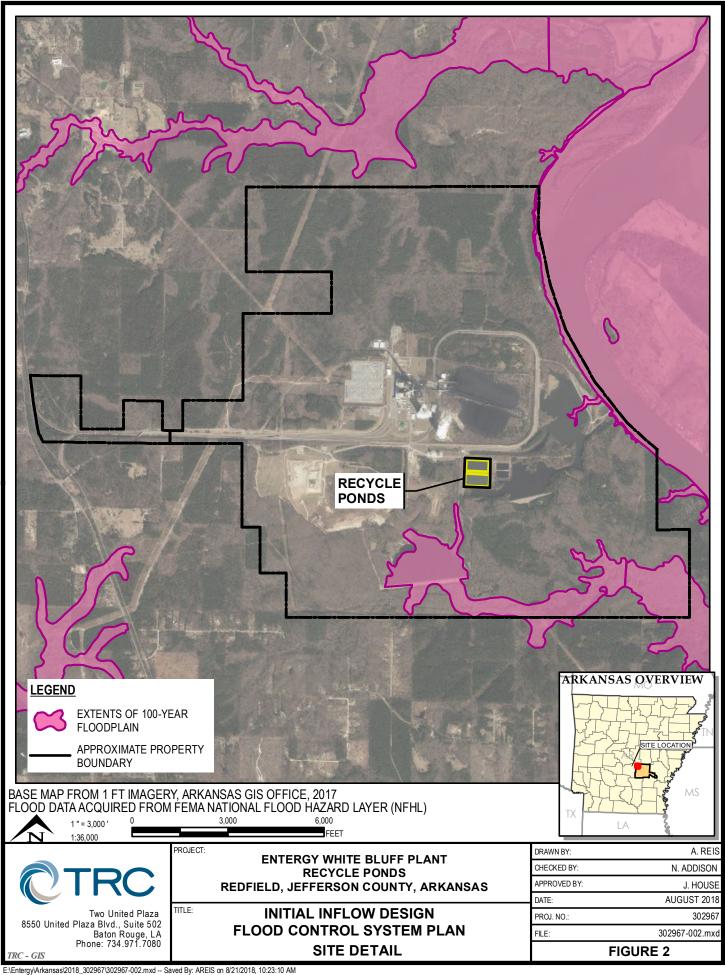
### 2.3 Pumping Capacity

It is assumed that the pump station is equipped with three pumps with a rated pumping capacity of 2,475 gallons per minute (gpm) at 320 feet of head for Unit 1 and Unit 2 based on the system flow diagram for ash disposal (Entergy, 1995). A calculation was performed to determine length of time required to remove the anticipated run-on due to a 100-year, 24-hour storm event assuming an outflow rate of 9,950 gpm, refer to Appendix B. The calculations resulted with a 15-hour time requirement to remove the anticipated storm water run-on. This calculation shows that pumping rates are sufficient in controlling water levels in the Ponds.

### 2.4 Conclusions

The Ponds meet the requirements of 40 CFR § 257.82 of adequately controlling the inflows and outflows of peak discharge at the Plant for the following reasons:

- The Ponds are located outside of the 100-year floodplain.
- The Ponds were adequately designed to withstand a 100-year, 24-hour storm event.
- The pumping rates are sufficient to control the water levels in the Ponds.



# Section 3 Notifications

October 2018

In accordance with 40 CFR § 257.105(g), Entergy will post to the Plant's Facility Operating Record (FOR) the Initial Inflow Design Flood Control System Plan. The Director of the ADEQ will be notified when documents are available as per 40 CFR § 257.106(g) and notices and documents will be placed on Entergy's CCR website consistent with 40 CFR § 257.107(g).

# Section 4 Amendment and Periodic Plan Revision

In accordance with 40 CFR § 257.82(c)(2), Entergy may amend this Initial Inflow Design Flood Control System Plan at any time. Specifically, Entergy will amend the inflow design flood control system plan whenever there is a change in the operation of the CCR unit that would substantially affect the written plan in effect. A periodic inflow design flood control system plan must also be prepared every 5 years from the completion date of this plan. Recordkeeping, Notification and Posting requirements of 40 CFR § 257.105, 257.106 and 257.107 will be followed.

### Section 5 References

- B&F Engineering, Inc. 2018. White Bluff Recycle Ponds Entergy Arkansas. October 4, 2018. Plan and Profile Drawing.
- B&F Engineering, Inc. 2018. White Bluff Recycle Ponds Entergy Arkansas. October 1, 2018. Topographic Survey Drawing.
- Chas. T. Main, Inc. 1981. White Bluff Steam Electric Station: 1977-1980 2800MW (Net) Installation – Units 1 Thru 4: Surge Pond Overflow: Holding Pond No. 2 & Water Treating Area – Section and Details. Rev. 2, 10/27/81. Chas. T. Main, Inc. Drawing No. 3022-01
- Chas. T. Main, Inc. 1983. White Bluff Steam Electric Station: 1977-1980 2800MW (Net) Installation – Units 1 Thru 4: Surge Pond Overflow: Holding Pond No. 2 & Water Treating Area – Plan. Rev. 8, 1/24/83. Drawing 3022-00.
- Entergy. 1995. White Bluff Units 1, 2, 3, & 4: Flow Diagram Ash Disposal (AW). Drawing No. 2020-00. January 4, 1995. Drawing.
- Federal Emergency Management Agency. 2009. Flood Insurance Rate Map: Jefferson County Arkansas Panel 25 of 600. Map Number 05069C0025D. Effective Date March 16, 2009. National Flood Insurance Program. Washington, D. C.
- ERM. 2018b. Summary of Site Visit and Review of CCR Structural Integrity Criteria Requirements for White Bluff Steam Electric Station Recycle Ponds. October 2018.

## Section 6 Certification

I, the undersigned Arkansas Professional Engineer, hereby certify that I am familiar with the technical requirements of 40 CFR § 257.82. I also certify that it is my professional opinion that, to the best of my knowledge, information, and belief, that the information in this inflow design flood control system plan is in accordance with current good and accepted engineering practice(s) and standard(s) and meets the technical requirements of 40 CFR § 257.82(c).

For the purpose of this document, "certify" and "certification" shall be interpreted and construed to be a "statement of professional opinion." The certification is understood and intended to be an expression of my professional opinion as an Arkansas Licensed Professional Engineer, based upon knowledge, information, and belief. The statement(s) of professional opinion are not and shall not be interpreted or construed to be a guarantee or a warranty of the analysis herein.

	9136	
R. Kent Nilsson, P.E.	State of Arkansas License Number	
Malsil	10/11/18	
Signature of Professional Engineer	Date	



# Appendix A Flood Insurance Rate Map

#### NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, users are encouraged to consult he Flood Profiles and Floodway. Data and/or Summany of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies the FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of order elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

able should be used for construction and/or noodprain mana-when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolate between cross sections. The floodways were based on hydraulic consideration with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this sirisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures.** Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Arkansas State Plane south zone (FIPSZONE 0002). The horizontal datum was NAD83. GRS1980 spheroid. Differences in datum, spheroid, projection or State Plane zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of the FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1986. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1989, visit the National Geodetic Survey website at http://www.ngs.nosa.gov/ or contact the National Geodetic Survey at the following address.

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 1315 East- West Highway Silver Spring, MD 20910-3282

Base map information shown on this FIRM was provided in digital format by Arkansas Geographic Information Office.

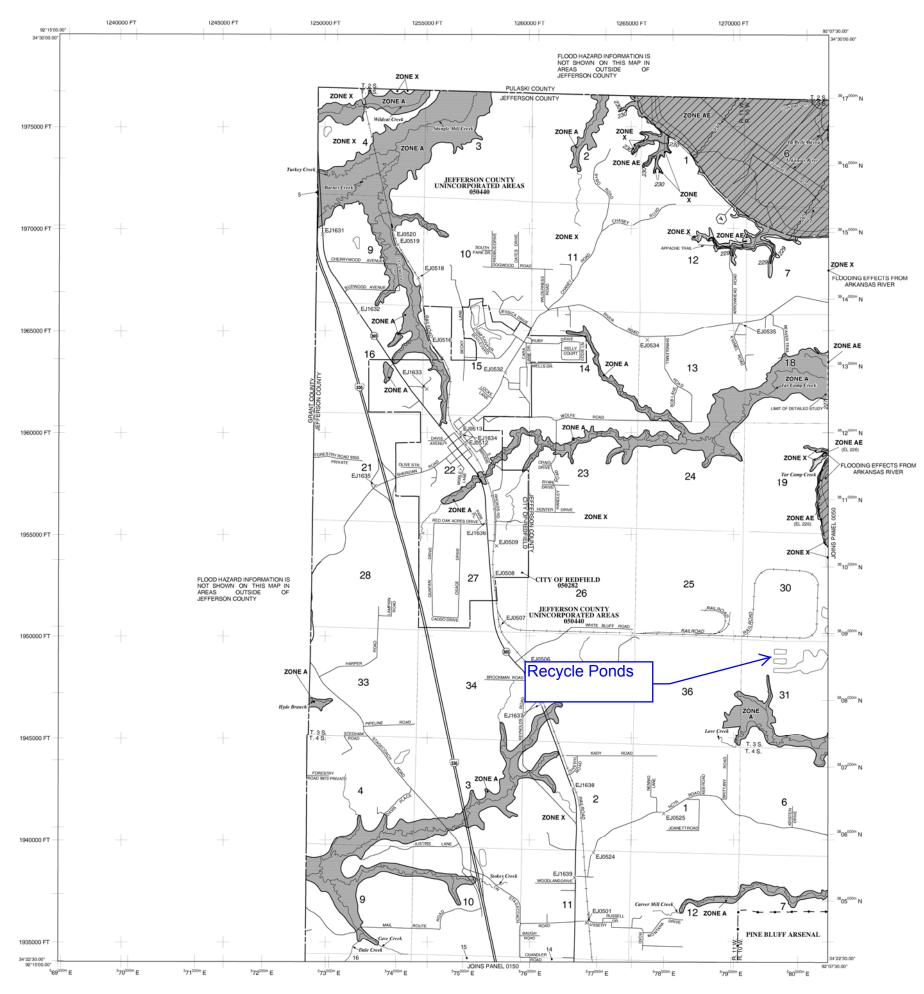
This map reflects more detailed and up-to-date stream channel configurations than tilizes shown on the previous FIRM for this jurisocition. The floodplaints and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hybritatic data may reflect stream channel distances that offer more what is shown on this map.

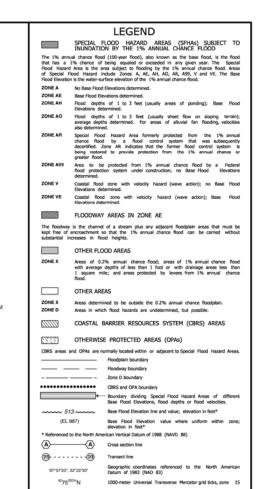
Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annoxations or de-annoxations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact the FEMA Map Service Center at 1-800-358-9616 for information on available products associated with this FIRM. Available products may include previously sissed Letters of Map Change, a Flood Insurance Study report, and/or oligital versions of this map. The FEMA Map Service Center may also be reached by Fax at 1-800-358-9620 and its website at http://www.mcs.fema.gov/.

If you have questions about this map or questions concerning the Flood insurance Program in general, please call - 877- FEMA MAP (1-877- 3 or visit the FEMA website at http://www.fema.gov/.









For community map revision history prior to countywide mapping, refer to the Community Map History table located in the Flood Insurance Study report for this jurisdiction.



050440 0025 D 050282 0025 D



MAP NUMBER EFFECTIVE DATE MARCH 16, 2009

Federal Emergency Management Agency

# Appendix B Storm Water Calculations

- Storm Water Run-On Calculation
- Pumping Time Estimate



#### NOAA Atlas 14, Volume 9, Version 2 Location name: Redfield, Arkansas, USA\* Latitude: 34.4164°, Longitude: -92.1326° Elevation: 279.97 ft\*\*

source: ESRI Maps
\*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

### PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) <sup>1</sup>										
Duration Average recurrence interval (years)										
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>0.445</b> (0.373-0.534)	<b>0.504</b> (0.422-0.605)	<b>0.598</b> (0.498-0.718)	<b>0.673</b> (0.558-0.811)	<b>0.773</b> (0.621-0.949)	<b>0.847</b> (0.669-1.05)	<b>0.919</b> (0.705-1.16)	<b>0.989</b> (0.733-1.28)	<b>1.08</b> (0.774-1.43)	<b>1.14</b> (0.804-1.54)
10-min	<b>0.652</b> (0.546-0.782)	<b>0.738</b> (0.617-0.886)	<b>0.875</b> (0.730-1.05)	<b>0.985</b> (0.818-1.19)	<b>1.13</b> (0.910-1.39)	<b>1.24</b> (0.979-1.54)	<b>1.35</b> (1.03-1.71)	<b>1.45</b> (1.07-1.88)	<b>1.58</b> (1.13-2.09)	<b>1.67</b> (1.18-2.25)
15-min	<b>0.796</b> (0.666-0.953)	<b>0.900</b> (0.753-1.08)	<b>1.07</b> (0.890-1.28)	<b>1.20</b> (0.997-1.45)	<b>1.38</b> (1.11-1.69)	<b>1.51</b> (1.19-1.88)	<b>1.64</b> (1.26-2.08)	<b>1.77</b> (1.31-2.29)	<b>1.93</b> (1.38-2.55)	<b>2.04</b> (1.44-2.75)
30-min	<b>1.19</b> (0.996-1.43)	<b>1.35</b> (1.13-1.62)	<b>1.61</b> (1.34-1.93)	<b>1.81</b> (1.50-2.18)	<b>2.08</b> (1.67-2.55)	<b>2.28</b> (1.80-2.83)	<b>2.47</b> (1.90-3.13)	<b>2.66</b> (1.97-3.44)	<b>2.89</b> (2.07-3.82)	<b>3.05</b> (2.15-4.11)
60-min	<b>1.59</b> (1.33-1.91)	<b>1.80</b> (1.51-2.16)	<b>2.14</b> (1.79-2.57)	<b>2.42</b> (2.01-2.92)	<b>2.79</b> (2.24-3.43)	<b>3.07</b> (2.42-3.82)	<b>3.34</b> (2.57-4.24)	<b>3.61</b> (2.68-4.68)	<b>3.96</b> (2.84-5.25)	<b>4.21</b> (2.96-5.67)
2-hr	<b>1.99</b> (1.67-2.37)	<b>2.25</b> (1.89-2.68)	<b>2.68</b> (2.25-3.19)	<b>3.03</b> (2.53-3.62)	<b>3.50</b> (2.83-4.28)	<b>3.86</b> (3.07-4.77)	<b>4.21</b> (3.26-5.31)	l (3.41-5	•	24-Hou
3-hr	<b>2.23</b> (1.89-2.65)	<b>2.53</b> (2.13-3.00)	<b>3.01</b> (2.54-3.58)	<b>3.42</b> (2.86-4.07)	<b>3.97</b> (3.24-4.85)	<b>4.40</b> (3.52-5.43)	<b>4.83</b> (3.75-6.08)	<b>5.27</b> (3 95-6,	torm Eve	ent 
6-hr	<b>2.68</b> (2.28-3.15)	<b>3.05</b> (2.59-3.59)	<b>3.67</b> (3.11-4.33)	<b>4.20</b> (3.54-4.97)	<b>4.94</b> (4.06-6.00)	<b>5.52</b> (4.45-6.78)	<b>6.12</b> (4.79-7.66	<b>6.73</b> (5.09-8.62)	<b>7.56</b> (5.53-9.90)	<b>8.20</b> (5.87-10.9)
12-hr	<b>3.17</b> (2.72-3.71)	<b>3.66</b> (3.12-4.27)	<b>4.46</b> (3.80-5.22)	<b>5.15</b> (4.37-6.05)	<b>6.13</b> (5.07-7.41)	<b>6.91</b> (5.60-8.44)	<b>7.71</b> (6.08-9.60)	<b>8.54</b> (6.50-10.9)	<b>9.67</b> (7.13-12.6)	<b>10.6</b> (7.60-13.9)
24-hr	(3.21-4.32)	(3.71-5.00)	5.30 (4.55-6.16)	<b>6.15</b> (5.25-7.18)	<b>7.37</b> (6.14-8.85)	8.34 (6.81-10.1)	9.34 (7.41-11.6)	10.4 (7.96-13.1)	11.8 (8.77-15.3)	12.9 (9.38-16.9)
2-day	<b>4.35</b> (3.76-5.00)	<b>5.01</b> (4.33-5.76)	<b>6.13</b> (5.29-7.07)	<b>7.10</b> (6.10-8.22)	<b>8.49</b> (7.12-10.1)	<b>9.60</b> (7.89-11.6)	<b>10.7</b> (8.59-13.2)	<b>12.0</b> (9.23-15.0)	<b>13.6</b> (10.2-17.5)	<b>14.9</b> (10.9-19.4)
3-day	<b>4.76</b> (4.14-5.46)	<b>5.46</b> (4.74-6.26)	<b>6.65</b> (5.75-7.63)	<b>7.66</b> (6.61-8.83)	<b>9.12</b> (7.68-10.8)	<b>10.3</b> (8.48-12.3)	<b>11.5</b> (9.22-14.1)	<b>12.8</b> (9.88-16.0)	<b>14.5</b> (10.9-18.5)	<b>15.8</b> (11.6-20.5)
4-day	<b>5.10</b> (4.44-5.82)	<b>5.83</b> (5.07-6.66)	<b>7.06</b> (6.13-8.08)	<b>8.11</b> (7.01-9.32)	<b>9.61</b> (8.11-11.4)	<b>10.8</b> (8.94-12.9)	<b>12.1</b> (9.69-14.7)	<b>13.3</b> (10.4-16.6)	<b>15.1</b> (11.4-19.3)	<b>16.5</b> (12.1-21.3)
7-day	<b>5.93</b> (5.19-6.72)	<b>6.73</b> (5.89-7.64)	<b>8.06</b> (7.04-9.18)	<b>9.20</b> (7.99-10.5)	<b>10.8</b> (9.15-12.7)	<b>12.1</b> (10.0-14.3)	<b>13.4</b> (10.8-16.2)	<b>14.7</b> (11.5-18.2)	<b>16.5</b> (12.5-20.9)	<b>17.9</b> (13.2-23.0)
10-day	<b>6.71</b> (5.89-7.58)	<b>7.55</b> (6.63-8.54)	<b>8.94</b> (7.83-10.1)	<b>10.1</b> (8.82-11.5)	<b>11.8</b> (9.99-13.7)	<b>13.1</b> (10.9-15.4)	<b>14.4</b> (11.6-17.3)	<b>15.7</b> (12.3-19.4)	<b>17.5</b> (13.3-22.1)	<b>18.9</b> (14.0-24.2)
20-day	<b>9.07</b> (8.02-10.2)	<b>10.0</b> (8.84-11.2)	<b>11.5</b> (10.2-13.0)	<b>12.8</b> (11.2-14.4)	<b>14.5</b> (12.4-16.8)	<b>15.8</b> (13.3-18.5)	<b>17.2</b> (14.0-20.5)	<b>18.5</b> (14.6-22.6)	<b>20.3</b> (15.4-25.3)	<b>21.6</b> (16.1-27.4)
30-day	<b>11.0</b> (9.77-12.3)	<b>12.1</b> (10.7-13.5)	<b>13.8</b> (12.2-15.5)	<b>15.2</b> (13.4-17.1)	<b>17.1</b> (14.7-19.7)	<b>18.6</b> (15.6-21.6)	<b>20.0</b> (16.3-23.7)	<b>21.4</b> (16.9-26.0)	<b>23.2</b> (17.7-28.8)	<b>24.5</b> (18.4-31.0)
45-day	<b>13.3</b> (11.9-14.8)	<b>14.7</b> (13.1-16.4)	<b>16.9</b> (15.0-18.8)	<b>18.7</b> (16.5-20.9)	<b>21.0</b> (18.0-23.9)	<b>22.7</b> (19.1-26.2)	<b>24.3</b> (19.9-28.6)	<b>25.9</b> (20.5-31.2)	<b>27.9</b> (21.4-34.5)	<b>29.3</b> (22.0-36.9)
60-day	<b>15.3</b> (13.6-16.9)	<b>17.0</b> (15.1-18.8)	<b>19.7</b> (17.5-21.8)	<b>21.8</b> (19.3-24.3)	<b>24.6</b> (21.1-27.9)	<b>26.6</b> (22.4-30.6)	<b>28.5</b> (23.4-33.5)	<b>30.3</b> (24.1-36.5)	<b>32.6</b> (25.0-40.1)	<b>34.2</b> (25.8-42.9)

Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

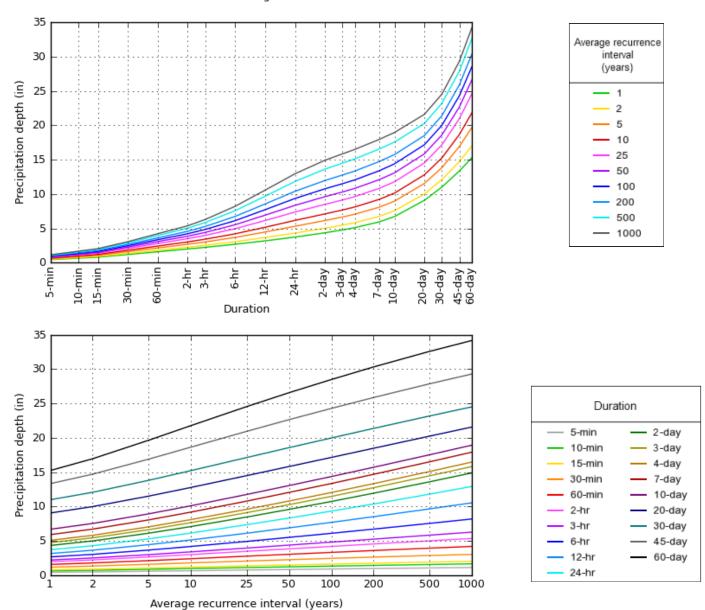
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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### PF graphical

PDS-based depth-duration-frequency (DDF) curves Latitude: 34.4164°, Longitude: -92.1326°



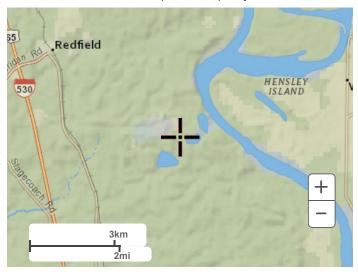
NOAA Atlas 14, Volume 9, Version 2

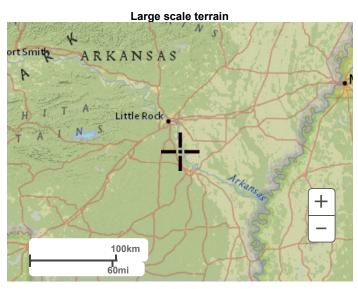
Created (GMT): Thu Aug 23 17:09:16 2018

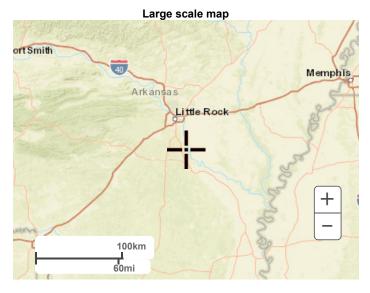
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### Maps & aerials

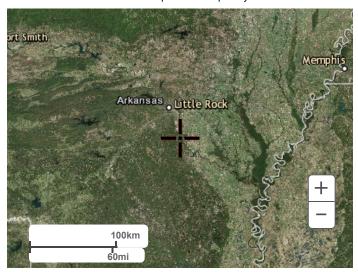
Small scale terrain







Large scale aerial



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National Weather Service
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**Disclaimer** 

## **Cut/Fill Report**

**Generated:** 2018-08-29 15:53:45

By user: JBell

J:\Entergy\302967 - White Bluff Plant\0000\Source

**Drawing:** Files\XREF\J:\Entergy\302967 - White Bluff Plant\0000\Source

Files\XREF\Pond Freeboard Volume.dwg

Volume Summary									
Name	Туре	Cut Factor	Fill Factor	2d Area (Sq. Ft.)	Cut (Cu. Yd.)	Fill (Cu. Yd.)	Net (Cu. Yd.)		
North Pond - Freeboard Volume	full	1.000	1.000	279631.43	0.00	19945.50	19945.50 <fill></fill>		
South Pond - Freeboard Volume	full	1.000	1.000	303650.46	0.00	21745.33	21745.33 <fill></fill>		

Totals				
	2d Area (Sq. Ft.)	Cut (Cu. Yd.)	Fill (Cu. Yd.)	Net (Cu. Yd.)
Total	583281.89	0.00	41690.83	41690.83 <fill></fill>

<sup>\*</sup> Value adjusted by cut or fill factor other than 1.0